

Development of Correlations for Compression Index

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Abstract

Compression index is one of the most important properties of soils widely used for estimation of settlement during design of foundations. In particular, for routine test requirements, limited laboratory facilities and preliminary evaluations, various equations are available in the literature to simply estimate the compression index using some soil index properties. However, applicability of these equations is often limited for the interested site due to their establishment from different databases. Thus, it is more realistic way to employ the correlations developed using local data of region, provided that data is available. In this viewpoint, this paper aims to develop correlations for the compression index dependent upon the soil index parameters of natural moisture content, liquid limit and initial void ratio for fine-grained soil of Baghdad city. From the developed correlations, it is obtained that the best estimations of compression index is correlated by liquid limit as single variable, and by liquid limit and initial void ratio as double variable. It is clear that use of the developed correlations is more suitable specifically for Baghdad region for preliminary evaluations of foundation settlement.

Keywords

Compression index,
Soil index property,
Liquid limit, Initial void
ratio, Correlation,
Foundation settlement.

Zemin Sıkışma İndeksi için Korelasyonların Geliştirilmesi

Özet

Sıkışma indeksi, temellerin tasarımında oturma hesapları için yaygın olarak kullanılan zeminlerin en önemli özelliklerinden biridir. Özellikle rutin deneyler, sınırlı laboratuvar koşulları ve ön değerlendirmeler için, literatürde zemin indeks özelliklerine bağlı olarak basitçe hesaplanabilen sıkışma indeksi denklemleri mevcuttur. Ancak, bu denklemler değişik verilerden oluşturulduğundan, çoğunlukla ilgililenen bölge için sınırlı kullanım sunmaktadır. Bu yüzden, bölgeye ait yerel veriler ile (eğer veri mevcutsa) geliştirilen korelasyonları kullanmak en gerçekçi yoldur. Bu doğrultuda, bu çalışmada Bağdat şehrine ait ince daneli zeminler için, zemin indeks özelliklerinden doğal su muhtevası, likit limit ve başlangıç boşluk oranına bağlı olarak sıkışma indeksi denklemlerinin geliştirilmesi amaçlanmıştır. Elde edilen denklemlerden, sıkışma indeksinin tek değişkenli ilişkilerde likit limit ile, ve çok değişkenli ilişkilerde ise likit limit ve başlangıç boşluk oranı ile birlikte kullanıldığında en iyi korelasyonları verdiği bulunmuştur. Geliştirilen korelasyonların temel oturması için yapılacak ön değerlendirmelerde özellikle Bağdat bölgesi için kullanılmasının çok daha uygun olduğu açıktır.

Anahtar kelimeler

Sıkışma indeksi, Zemin
indeks özelliği, Likit
limit, Başlangıç boşluk
oranı, Korelasyon,
Temel oturması.

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1. Introduction

An important phase in the selection and design of an appropriate foundation system of constructions is to evaluate the compressibility characteristics of soil. One of the important characteristics of soil compressibility that has been commonly used in the calculation of consolidation settlement of foundations is the compression index. It has been experimentally determined from oedometer tests in accordance with the procedures described in

technical standards in detail (ASTM D2435-11, 2011; ASTM D4186-12, 2012). However, considering the oedometer tests for this purpose from in-situ sampling to testing phase mostly requires a lot of time and becomes cost-prohibitive for the civil engineering projects particularly for the routine test requirements and limited laboratory facilities. This has been a significant concern specifically for small projects (Giasi *et al.* 2003). Thus, numerous attempts have been made by several researchers to correlate the compression

index of soil specifically dependent upon some simple index properties that mostly include moisture content, liquid limit, plasticity index, shrinkage index, initial void ratio, etc. A wide collected number of these correlations have been summarized in the past works (Burland, 1990; Giasi et al. 2003; Onyejekwe et al. 2015). The interested readers are referred to the past works for a detailed description. In comparison with the consolidation test, obtaining the soil index properties has the advantages of being relatively inexpensive testing that does not require much time or any complex systems. Therefore, it could be very useful in practice to use the existing empirical correlations of compression index for estimation of settlement. However, due to their developments based on the site-specific data and/or widely sourced data, the available correlations of compression index in the literature could not be always assumed to have a general validity for other regions of local sites (Lav and Ansal 2001; Giasi et al. 2003; Onyejekwe et al. 2015). Alternatively, the best way is to develop the correlations of compression index with the data of local site of the interested region, provided that data is available. In this way, it is the most possible to obtain the realistic results for settlement estimation by means of correlations.

For the issue of concern arised above, this paper aims to develop the correlations of compression index versus soil index properties for the soils of Baghdad city along Tigris River, using its local data. In this viewpoint, it is observed that the correlations for compression index for Baghdad city have not been sufficiently studied up to now. Baghdad is the largest city of Iraq located along the Tigris River including poor soils of alluvial and fluvial deposits in the middle Mesopotamian plain. Due to a wide destroyed during the last decades in the city, an increase demand for new constructions has been expected for settling, infrastructure and industrial facilities within urban planning purposes. Thus, it is believed that the proposed correlations of compression index in this paper could

significantly contribute at the estimation of consolidation settlement for foundation design of constructions at the Baghdad region, particularly for preliminary evaluations.

2. Material and Method

2.1. Some available equations for compression index

It would be useful to review available equations of compression index in literature to understand the independent parameters of compression index as well as their mathematical form. The equations could also be beneficial to compare the developed correlations in this study. In the review of the equations, it is observed that, in most of cases, the compression index (C_c) is expressed by establishment with the single index property, primarily including moisture content (w_n), liquid limit (LL) and initial void ratio (e_0). This is matching with the observation of past work (Lav and Ansal 2001). Thus, this study will mainly be focused on the derivation of the correlations for the compression index with single parameter using these three basic index soil properties (i.e., i) $C_c = f(w_n)$, ii) $C_c = f(LL)$, iii) $C_c = f(e_0)$). But, it is also included the double parameter using LL and e_0 recommended in some earlier works (i.e., iv) $C_c = f(e_0, LL)$) (Li and White 1993; Cherubini and Giasi 2000). Table 1 presents some of the equations available in the literature, with their applicability of soils. These equations have already been reviewed for performance comparisons in the past works (Giasi et al. 2003; Onyejekwe et al. 2015). Thus, it can be considered that they indicate a potential that is well correlated with compressibility.

As discussed some of the equations in Table 1, the equation of Terzaghi and Peck (1967) dependent upon the liquid limit among the evaluated correlations was found suitable to propose for blue and varicoloured clays outcropping in the southern part of the Bradanic Foredeep, in a past study

investigating appropriate correlations in the literature (Giasi et al. 2003). In the study of Onyejekwe et al. (2015), the equation of Azzouz et al. (1976) with liquid limit was found overall best to propose for southeast region of Missouri soils, while the equation of Azzouz et al. (1976) with moisture content was proposed overall best one for other regions of Missouri soils. The findings from the past works (Giasi et al. 2003; Onyejekwe et al. 2015) reveal that the applicability of the multitude equations developed up to now could become questionable for the same site of interested region due to their development of different databases. Thus, as in this study, the most useful way to benefit from the correlations of compression index is to derive self site-specific correlations using local data, as stated earlier.

As can be seen from Table 1, the correlations in the most cases are linear-regression-based, which will be employed in this study, with the independent parameter of index property. The degree of compressibility of clay soils is mostly connected to the size of the surface of solid particles, their mineralogy, stress state and water interaction. Since the index properties, partially or fully, represent this nature of compressibility, they are relatively preferred for estimating compression index in an easily way of determination (Giasi et al. 2003). For the index property of liquid limit, it is reported that the liquid limit is a measure of the quantity of water attracted by the solid particles for a given value of undrained shear strength (Al-Khafaji and Andersland 1992). But, it should be emphasized that all Atterberg limits are mostly considered as the representative parameters specifically for the behavior induced mineralogical composition for remolded soil (Skempton 1944) or normally consolidated one (Terzaghi and Peck 1967). In view of stress state, Tsuchida (1999) notes that when consolidating a sample with a pressure much larger than the preconsolidation stress, a good correlation between the liquid limit and compressibility is identified. Here, it is important to give a particular attention that soils

having same liquid limit but different plastic and shrinkage limits could result in different compressibility behavior, since the liquid limit cannot be fully representative of plasticity characteristics (Sridharan and Nagaraj 2000). Alternatively, some correlations were developed based on shrinkage and plasticity index (Sridharan and Nagaraj 2000). However, both of them are very limited and not found a wide usage in practice compared to the ones with liquid limit.

As for the initial void ratio being the independent parameter of compression index equations, it is reported that in addition to representation of intrinsic soil properties, it takes geological stress history of soil structure into account, which is very important during the estimation of settlement. This is an important parameter particularly for the definition of initial conditions of undisturbed samples (Giasi et al. 2003). This implies that for the behavior of an undisturbed sample it is necessary to develop the equations based on more than one parameter, in order to take both the initial state and the soil composition into account (Giasi et al. 2003). This could also be a general recommendation for all samples. Thus, the equations using multiple parameters could provide a more accurate estimate of the compression index, in particular based on the parameters of e_0 and LL (Li and White 1993; Cherubini and Giasi 2000). However, here it is important to inform from the study of Onyejekwe et al. (2015) that the equations of compression index with double independent parameter including the initial void ratio were not found overall best among the investigated equations for the interested site, compared with the equations with single parameter. This clearly indicates the importance of development of self site-specific correlations using local data in order to understand the effect of index properties well.

In this paper, some of the widely considered equations in literature reported in Table 1 have also been compared with the ones derived in this

paper. The attention has been given to the performances due to correlation coefficient (R) and root mean square error (RMSE) defined by Eq.1, as follows:

$$RMSE = \left(\frac{\sum_{i=1}^N (C_{c_{meas}} - C_{c_{est}})^2}{N} \right)^{1/2} \quad (1a)$$

$$R = \left(\frac{\sum_{i=1}^N (C_{c_{meas}})^2 - \frac{\sum_{i=1}^N (C_{c_{meas}} - C_{c_{est}})^2}{N}}{\sum_{i=1}^N (C_{c_{meas}})^2} \right)^{0.5} \quad (1b)$$

where N= number of data, $C_{c_{meas}}$ = the compression index measured directly from laboratory tests and

$C_{c_{est}}$ = the compression index estimated from empirical correlation. It is well known that obtaining lower error with higher correlation is the goal of performance evaluation of correlation. The comparisons clearly help to see whether the available equations (Table 1) could be alternatively proposed for the region. However, it is obvious that it is necessary to compare the results obtained using other equations. This is beyond the scope of the work presented in this paper that remains a topic of performance investigation of available correlations for Baghdad soils in detail to be in a separate work.

Table 1. Some of available correlations for compression index

Equation	Comment	Reference
$C_c = 0.01w_n - 0.05$	All clays	Azzouz et al. (1976)
$C_c = 0.01w_n - 0.07549$	All clays	Rendon-Herrero (1983)
$C_c = 0.007LL - 0.07$	Remolded clays	Skempton (1944)
$C_c = 0.009LL - 0.09$	All clays	Terzaghi and Peck (1967)
$C_c = 0.006LL - 0.054$	All clays with LL<100	Azzouz et al. (1976)
$C_c = 0.008LL - 0.096$	All clays	Sridharan and Nagaraj (2000)
$C_c = 0.29e_0 - 0.0783$	Inorganic soils	Hough (1957)
$C_c = 0.75e_0 - 0.375$	Soils with low plasticity	Sowers (1970)
$C_c = 0.4e_0 - 0.1$	All natural soils	Azzouz et al. (1976)
$C_c = 0.156e_0 + 0.0107$	All clays	Bowles (1979)
$C_c = 0.00058LL + 0.411e_0 - 0.156$	All clays	Al-Khafaji and Andersland (1992)

2.2. Data used for development of correlations

The data used in this study consist of the parameters C_c , w_n , LL and e_0 belonging to the testing results of fine-grained soils (clay) of

Baghdad city. The data were obtained from a previous work ([Alhashemy, 2015](#)), where it is collected from ANDREA Engineering Testing Laboratory ([ANDREA, 2016](#)). The testings of

oedometer and index property were conducted for the Baghdad city, specifically along the Tigris River for a variety of constructions. As regards the methodology used to obtain the compression index and the index properties of soils, it should be emphasized that all testings were performed under similar conditions and using the same technique. The data used for development of correlations in this study includes the number of 69 samples. However, it is important to note that as the database is updated with more supplied data, the development of correlations should be reconsidered in future. Table 2 summarizes the data in some descriptive statistics of variables. Fig.1 illustrates the data in their frequency histogram with normal distribution curve. As shown from the Fig.1 that the frequencies of correlation parameters mostly follow well with the normal distribution curve. This implies that the database is statistically suitable for correlation considerations.

As for the general description of the soils for the Baghdad city (Alhashemy, 2015), lean to fat silty clay with soft to medium or stiff consistencies and fine-grained mixtures of silt and clay with sand and gravel in alternate thicknesses are mostly encountered at the near surface specifically in the Tigris banks. This stratum is overlying with the medium to dense silty sand and fine gravel layers. In some parts, the coarse-grained layer extends down to 17m-18m. The groundwater table varies from 0.9m to 7m within the boundary in different part of city. In accordance with Unified Soil Classification System, the region has the fine grained soils that can be classified as CH and CL in both. Plasticity index of soils changes approximately from 14 to 40. The soils passing the sieve size of 0.075mm are in the percentage from 78% to 96%. The unit weight of soils is in the range that approximately varies from 14kN/m³ to 17kN/m³.

Table 2. Data used in the study in some descriptive statistics of variables.

Parameter	Cc	w _n	LL	e ₀
Minimum	0.12	17.1	32	0.511
Maximum	0.28	30.4	62	0.883
Mean	0.19	24.1	44	0.711
Std. Deviation	0.03	3.2	6	0.085

Cc=compression index
w_n=natural moisture content (%)
LL=liquid limit
e₀=initial void ratio

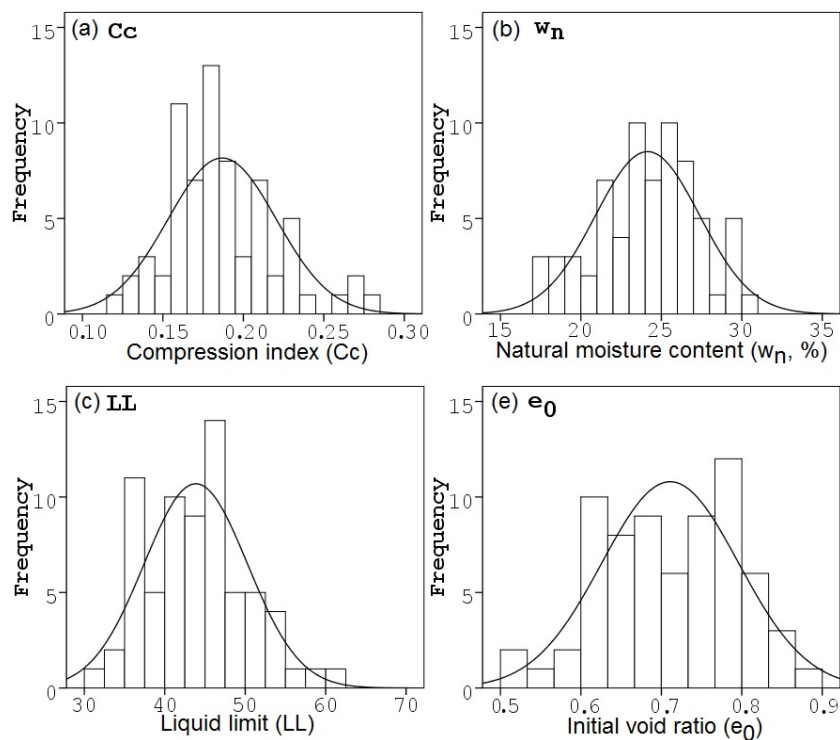


Figure 1. Data used in the study in the frequency histogram.

3. Results and Discussions

The developed correlations of C_c in this study separately established with the independent parameters of w_n , LL and e_0 as the single variable correlations and with the parameters LL and e_0 together as the double variable correlation are presented in Table 3. The performances of the developed correlations in RMSE and R are given in Table 4 comparing with the performances of the available equations (given in Table 1). The performances of the correlations through the estimated C_c results ($C_{c_{est}}$) versus laboratory measured C_c ones ($C_{c_{meas}}$) have been graphically compared in scattering curve by considering the precise of equations in accordance with perfect line (i.e., $y=x$), as given in Fig.2, regarding the independent parameters of w_n (Fig.2a), LL (Fig.2b), e_0 (Fig.2c) and e_0 , LL (Fig.2d).

As shown from the performances of the developed correlations in Table 3, the best estimation of C_c (i.e., lower RMSE with higher R) appears to be obtained with the input inclusion of LL

(RMSE=0.017, R=0.87) as single variable correlation and with the inputs of LL and e_0 (RMSE=0.016, R=0.87) as double variable correlation, in a strong performance. They are followed by the correlations with the input of w_n (RMSE=0.028, R=0.53) in a moderate performance and with the input of e_0 in a poor performance (RMSE=0.033, R=0.21). The best estimations indicate that the mineralogical composition of soil only (due the correlation with LL) and/or the mineralogical composition and stress history with together (due to the correlation with LL and e_0) become prominent for development of correlations of C_c for Baghdad soils. This soil behavior on C_c relatively supports the suggestions of earlier works (Li and White 1993; Cherubini and Giasi 2000). On the other hand, the stress history of soil only (due to the correlation with e_0) appears to become weak. This may be attributed to the sampling disturbance prior or during testing. As compared with the past work in the viewpoint of variable performance (Onyejekwe et al. 2015), the performance of the correlation with LL is found in a good matching as a best estimation. While the correlation with e_0 and

LL produces better performance, the correlation with w_n performs less. For the correlation with e_0 , similar poor performances are encountered in the past work. On the other hand, a comprehensive study (Tiwari and Ajmera 2012) on the intrinsic consolidation characteristics of soil specimens covering a wide range of activities from highly active to less active clay minerals prepared with consistent initial moisture content found a strong correlation of C_c with e_0 . From the comparisons, it can be said that all variations of the performances could be attributed to the fact that the estimations are induced by the site-specific local data due to soil type as well as sampling of testing. Although many researchers starting from Skempton (1944) conducted one dimensional consolidation tests on various natural and reconstituted soil specimens in order to develop correlations between C_c and other index properties of soil, the sample preparation methods used were different (Tiwari and Ajmera 2012).

As for the performance comparison with available equations (Table 4), except the correlation with LL and e_0 , all performances of the developed equations are found similar to the available ones. This similar response was actually expected, because all the compared available equations have been selected on the consideration of having a good estimation potential of C_c due to the findings in past studies (Giasi et al. 2003; Onyejekwe et al. 2015). At a first sight, the performances may indicate that the available correlations (except Al-Khafaji and Andersland 1992) may be alternately proposed for estimation of C_c . However, here it should be emphasized that for final decision of best correlations in the comparisons, some ranking calculations (Cherubini and Orr 2000) that determine accuracy and precise of estimations are recommended. This is a topic of separate investigation. But, in place of ranking evaluations in this study, decision on which correlation is better and/or which available correlation is alternately proposed for the region in place of developed correlations will be made considering the perfect

fit (i.e., $y=x$) in the scattering plots due to the estimated C_c results versus laboratory measured C_c results with respect to the independent parameters (Fig.2).

As shown from Fig.2, all correlations result in different performances that represent the precision of estimation in accordance with the perfect curve (i.e., $y=x$). It is well known from a scattering curve (Cherubini and Orr, 2000) that as the data points are accumulated along the perfect curve, the estimation of correlation indicates better fit of performance in a better precision. As concerned with the correlations with the parameter w_n (Fig.2a), it is observed that the correlation developed in this study presents better fit of precision with the perfect curve. This is followed by Azzouz et al. (1976) that can be alternately proposed for estimation of C_c for the region. Regarding the correlations with LL (Fig.2b), it is clear that the correlation developed in this study performs best along the perfect line. This is followed by Azzouz et al. (1976) and then Skempton (1944), which indicate that they can be alternately proposed for estimation of C_c . For the correlations with e_0 (Fig.2c), it is observed that all correlations have poor accumulation in precision along the perfect line. Nonetheless, Azzouz et al. (1976) seems to perform better than the others, also including the correlation developed in this study that seems to be not suitable for estimation of C_c . From this, it can be said Azzouz et al. (1976) could be proposed for estimation of C_c dependent upon the parameter e_0 . As for the correlations with LL and e_0 (Fig.2d), it is clear that the developed correlation in this study relatively follow best with the perfect curve. Alternately, the scattering of data in Al-Khafaji and Andersland (1992) appears reasonable despite of its low correlation coefficient. As compared with the past work (Onyejekwe et al. 2015), the success of the correlations Azzouz et al. (1976) with w_n and with LL discussed the results above seems relatively consistent. Among the correlations between C_c and liquid limit, it is reported (Tiwari and Ajmera 2012)

that the correlation [Skempton \(1944\)](#) provides a closer approximation for the low activity soils in comparison to the correlation [Terzaghi and Peck's \(1967\)](#). However, these two correlations are

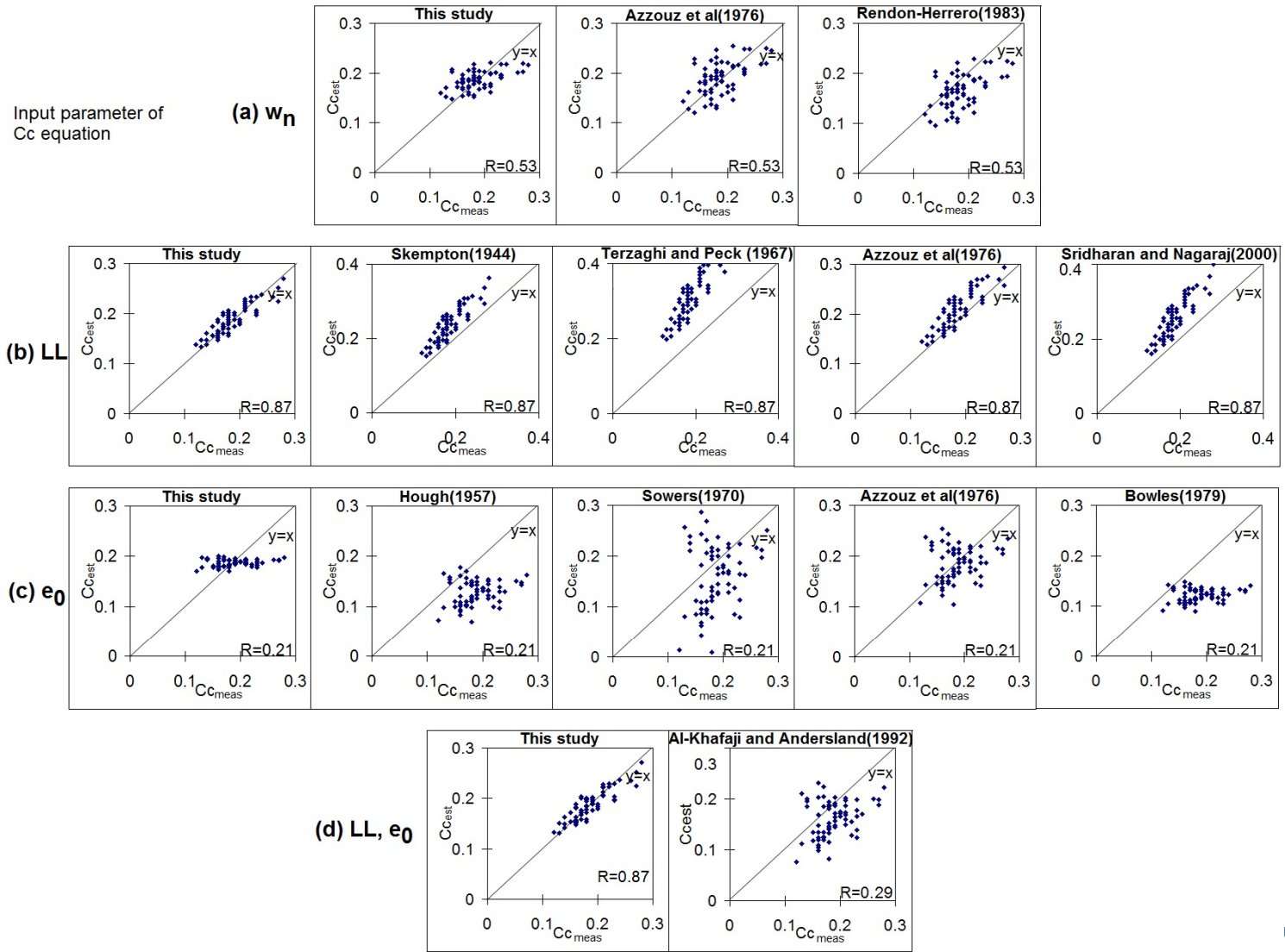
reported to underestimate C_c for soils having high activity that indicates the importance of the mineralogical composition.

Table 3. Developed correlations of C_c for fine-grained soils (clay) of Baghdad city.

Independent parameter	Equation	RMSE	R
w_n	$Cc = 0.00553w_n + 0.05321$	0.028	0.53
LL	$Cc = 0.00454LL - 0.01246$	0.017	0.87
e_0	$Cc = 0.08358e_0 + 0.12739$	0.033	0.21
LL, e_0	$Cc = 0.004483LL + 0.028871e_0 - 0.03029$	0.016	0.87

Table 4. Performances of the developed correlations in this study and available correlations.

Independent parameter	Equation	RMSE	R
w_n	This study	0.028	0.53
	Azzouz et al. (1976)	0.032	0.53
	Rendon-Herrero (1983)	0.038	0.53
LL	This study	0.017	0.87
	Skempton (1944)	0.055	0.87
	Terzaghi and Peck (1967)	0.122	0.87
	Azzouz et al. (1976)	0.029	0.87
	Sridharan and Nagaraj (2000)	0.073	0.87
e_0	This study	0.033	0.21
	Hough (1957)	0.069	0.21
	Sowers (1970)	0.071	0.21
	Azzouz et al. (1976)	0.042	0.21
	Bowles (1979)	0.073	0.21
LL, e_0	This study	0.016	0.87
	Al-Khafaji and Andersland (1992)	0.048	0.29



- 2 **Figure 2.** Estimated Cc results ($C_{c_{est}}$) versus laboratory measured Cc ones ($C_{c_{meas}}$), regarding Cc equation with the independent parameter: a) w_n ,
3 b) LL, c) e_0 and d) LL, e_0 . $y=x$ indicates perfect fit.

4. Conclusions

A database consisting of 69 data sets containing the consolidation and index parameters of C_c , w_n , LL and e_0 was used to develop correlations for estimating of compression index for fine-grained soils of Baghdad city. The correlations were developed regarding the independent parameters i) w_n , ii) LL, iii) e_0 and iv) LL, e_0 (Table 3). The developed correlations have also been compared with the some available correlations in order to understand their performance and alternate usage (Table 4, Fig.2). Based on the findings and observations from the study, the following conclusions could be drawn for estimation of C_c for fine-grained soils of Baghdad city:

- 1) The best correlations were developed due to the independent parameters with LL (RMSE=0.017, R=0.87) and with LL and e_0 (RMSE=0.016, R=0.87). They were followed by the correlations with w_n (RMSE=0.028, R=0.53) and with e_0 (RMSE=0.033, R=0.21).
- 2) All the developed correlations, except the correlation with LL and e_0 , are found to show similar performances (RMSE, R) with the available correlations. The performance of the correlation with LL and e_0 is found better than the one of the available correlation.
- 3) Based on the performances of RMSE, R together with the precision due to scattering of data along perfect fit (i.e., $y=x$), all the developed correlations, except the correlation with e_0 , could be proposed for estimation of C_c .

4) Among the available correlations, the correlation with w_n due to Azzouz et al. (1976), the correlations with LL due to Azzouz et al. (1976) and Skempton (1944), the correlation with e_0 due to Azzouz et al. (1976) and the correlation with LL and e_0 due to Al-Khafaji and Andersland (1992) are found to follow better performance that could be alternately considered for estimation of C_c .

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